



Simulation of non-stationary stochastic ground motions based on recent Italian earthquakes

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Abstract

This work presents an up-to-date model for the simulation of non-stationary ground motions, including several novelties compared to the original study of Sabetta and Pugliese (Bull Seism Soc Am 86:337–352, 1996). The selection of the input motion in the framework of earthquake engineering has become progressively more important with the growing use of nonlinear dynamic analyses. Regardless of the increasing availability of large strong motion databases, ground motion records are not always available for a given earthquake scenario and site condition, requiring the adoption of simulated time series. Among the different techniques for the generation of ground motion records, we focused on the methods based on stochastic simulations, considering the time-frequency decomposition of the seismic ground motion. We updated the non-stationary stochastic model initially developed in Sabetta and Pugliese (Bull Seism Soc Am 86:337–352, 1996) and later modified by Pousse et al. (Bull Seism Soc Am 96:2103–2117, 2006) and Laurendeau et al. (Nonstationary stochastic simulation of strong ground-motion time histories: application to the Japanese database. 15 WCEE Lisbon, 2012). The model is based on the S-transform that implicitly considers both the amplitude and frequency modulation. The four model parameters required for the simulation are: Arias intensity, significant duration, central frequency, and frequency bandwidth. They were obtained from an empirical ground motion model calibrated using the accelerometric records included in the updated Italian strong-motion database ITACA. The simulated accelerograms show a good match with the ground motion model prediction of several amplitude and frequency measures, such as Arias intensity, peak acceleration, peak velocity, Fourier spectra, and response spectra.

Keywords Stochastic simulation · Ground motion predictive models · Omega-square model · Time envelope function · Non-stationary accelerograms · Target response spectrum

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1 Introduction

With the increase of computational power and the advent of performance-based earthquake engineering, nonlinear dynamic analyses in the time domain are more often employed for the seismic assessment of new and existing civil constructions, including strategic and base-isolated structures, high ductility and irregular structures, coupled soil-structure systems. In the last years, several databases containing many natural ground motion records were created and updated (Chiou et al. 2008; Bozorgnia et al. 2014; Luzi et al. 2016) and many authors proposed criteria and methods for the record selection (Katsanos et al. 2010; Iervolino et al. 2010a; Baker and Lee 2018). Nevertheless, as the number of ground motion records of strong earthquakes in near source conditions is still limited and they cannot always match the given scenarios, synthetic ground motions are often used in the evaluation of seismic demand of structures. The simulated ground motions must be compatible with a given design response spectrum and the assumed earthquake scenario, usually described in terms of magnitude, distance, site geological conditions and focal mechanism.

A detailed review of the different techniques for predicting earthquake ground motions for engineering purposes is provided in Douglas and Aochi (2008).

The filtering and windowing of white noise, as the classic stochastic stationary procedure of SIMQKE (Gasparini and Vanmarcke 1976) and the analysis of the nonlinear dynamic response in the Kanai-Tajimi model (Kanai 1961; Lin and Yong 1987) were particularly used in the past. These methods, based on the random-vibration theory (Boore and Joyner 1984), provide accelerograms whose response spectra match the target spectrum. The spectral matching procedures are carried out in the frequency domain using a power spectral density function, the selection of which is the key issue and represents the main difference among various generation procedures. The main weakness of this kind of approach is that the time series are stationary in frequency and completely unrelated to any geophysical parameter.

A second category of methods relies on physics-based deterministic methods, where the ground motion is modelled by convolving source, path, and site effects (Spudich and Hartzell 1985). The kinematic models characterize the rupture process in terms of fault displacement as a function of time and location. The response at the site is calculated convolving the source function with the Green's functions that represent the ground motion deriving from a point dislocation at a specific position on the target fault. (Kamae et al. 1998; Hartzell et al. 2005). These methods require a detailed knowledge of the source rupture characteristics, and are limited in frequency, generally up to 1–2 Hz.

The dynamic models (where the rupture process is simulated by considering the stress conditions) represent a step up in complexity from kinematic models and have the advantage of proposing various possible rupture scenarios of different magnitudes for a given seismotectonic situation. They are based on the numerical solution of the elasto-dynamic equations in a discretised continuum and are represented by finite-difference and finite-elements methods (Smerzini and Villani 2012; Paolucci et al. 2018). The physics-based numerical simulations are also limited in frequency, demand high computational resources, and require a reliable 3D model that is hardly available in practice.

Another approach to ground motion simulation consists in stochastic methods (Hanks 1979; Hanks and McGuire 1981) that combine seismological models of the spectral amplitude of ground motion with the engineering notion that the high frequency motion is basically random. These methods grew out of the consideration that large part of the strong ground shaking, usually associated with the arrival of S waves,

appears incoherent and essentially random in nature. Furthermore, the Fourier amplitude spectrum (FAS), predicted by the Brune (1970) source model and often observed in actual earthquakes, is approximately constant between the corner frequency f_c and f_{\max} (frequency at which the logarithm of the FAS starts to decay for increasing frequencies). The strong ground motion can be approximated by finite duration (arrival of S waves), band limited ($f_c - f_{\max}$) Gaussian white noise. Under this assumption, by fixing the spectral amplitude and then generating different arrays of phases (random numbers uniformly distributed between 0 and 2π) it is possible to simulate acceleration time series looking similar in the frequency content but different in the details. This method was originally developed by Boore (1983) and made popular by the Stochastic-Method SIMulation (SMSIM) computer code (Boore 2003). An extension to finite faults is represented by EXSIM, a stochastic finite-source simulation algorithm (Boore 2009; Atkinson and Assatourians 2014). The application of this kind of models requires the knowledge of several parameters characterizing the source process and the wave propagation. The variability of the motion is considered by the random phase generation and the frequency content is assumed stationary with time.

Some recent studies compare the impact of different techniques on the results of the final engineering analysis. Schwab and Lestuzzi (2007) carried out nonlinear analyses on single-degree-of-freedom (SDOF) systems using time-series produced by the stationary procedure of SIMQKE (Gasparini and Vanmarcke 1976) as well as semiempirical non-stationary stochastic simulations (Sabetta and Pugliese 1996). They show that the classic stationary procedure leads to a significant underestimation of the ductility demand compared to natural accelerograms, whereas the non-stationary procedure performs much better (Iervolino et al. 2010b).

Among the above listed techniques for generating accelerograms, in this study we focused on methods based on stochastic simulations, considering the time–frequency decomposition of the seismic ground motion (Laurendeau 2013; Causse et al. 2014; Hong and Cui 2020). We have chosen to update the non-stationary stochastic model initially developed by Sabetta and Pugliese (1996) and modified by Pousse et al. (2006) and by Laurendeau et al. (2012). This method has the advantage of being simple, fast, and considering the basic concepts of seismology (Brune’s source model, realistic time envelope function, non-stationarity in frequency, ground-motion variability). The generation of the simulated accelerograms depends on few input parameters: moment magnitude, source to site distance, shear wave velocity in the uppermost 30 m at the site, and style of faulting. This means that the end-user needs to know only some basic parameters of the target earthquake scenario. The four model parameters required for the simulation are obtained from regression analyses on a strong motion dataset of shallow active crustal events in Italy (Lanzano et al. 2019) and are embedded in the simulation code: Arias intensity (IA), significant duration (DV), central frequency (F_c), and frequency bandwidth (F_b). The recent development of large strong-motion databases for Europe as the Engineering Strong Motion database ESM 2.0 (<https://esm-db.eu>; Luzi et al. 2020) and the Italian ACcelerometric Archive ITACA 3.1 (<http://itaca.mi.ingv.it>; D’Amico et al. 2020), was a key motivation for the updating of the stochastic method. The purpose of this study is to provide the earthquake engineering community with an improved stochastic model for the simulation of non-stationary accelerograms, giving a good fit with the target response spectrum even for a small number of simulations. Time-domain simulations are derived from a time–frequency decomposition of the signal and depend on the above-mentioned parameters obtained from the Ground Motion Model (GMM) by Lanzano et al. (2019). It follows that the results presented herein are specific for Italy, although the approach is general and

can be adapted to other databases if predictive equations for Arias intensity and significant duration are available.

2 Non-stationary stochastic ground motion model

We start from the model proposed by Sabetta and Pugliese (1996), hereafter SP96, a simulation of non-stationary time series based on the summation of Fourier series with random phases and time-dependent coefficients. The coefficients of the Fourier series are obtained from a frequency-time decomposition of the signal that, according to the definition given by Stockwell et al. (1996), is named S-transform. The S-transform provides frequency-dependent resolution with referenced phase information and the sum of the coefficients of the S-transform over time equals the Fourier coefficients. Unlike the continuous wavelet transform, the S-transform produces a time–frequency representation instead of a time-scale representation (Cui and Hong 2020).

The S-transform of a signal $x(t)$, such as a ground-motion record, is defined as (Stockwell et al. 1996):

$$S(f, \tau) = \int_{-\infty}^{\infty} x(t)w(f, \tau - t)e^{-i2\pi ft} dt \quad (1)$$

in which $S(f, \tau)$ is the S-transform of $x(t)$, f and t are frequency and time, and τ is the center of the window function $w(f, \tau - t)$. A frequently selected window is a Gaussian function represented by:

$$w(f, t) = \frac{|f|}{\sqrt{2\pi\kappa}} \exp\left(-\frac{f^2 t^2}{2\kappa^2}\right) \quad (2)$$

in which κ is a parameter that controls the number of oscillations in the effective width of the window. In our application, we set κ equal to 1. The window width for a given κ is inversely proportional to the frequency. Figure 1 shows the application of the S-transform to the ground motion recorded at the station of CSC during the Central Italy earthquake (Mw=6.5) of October 30, 2016. It can be noted as the high frequencies decrease with increasing time.

If we take the square of the S-transform, it becomes a natural extension of the power spectrum to the non-stationary case and it is constituted by a series of Power Spectral Densities (PSDs), calculated at different times.

$$Xs(t, f) = |S(t, f)|^2 \quad (3)$$

The function $Xs(t, f)$ upgrades the frequency-time decomposition used in SP96 and called Physical Spectrum (PS) which had a moving time window represented by a Gaussian function with a fixed length of 2.5 s. The PSDs can be fitted with a lognormal function defined through three parameters derived from the theory of the spectral moments (Vanmarcke 1980; Lai 1982). These parameters are the average total power P_a , corresponding to the area under the PSD; the central frequency F_c , giving a measure of where the PSD is concentrated along the frequency axis; and the frequency bandwidth F_b , corresponding to the dispersion of PSD around the central frequency. By inserting the time dependence

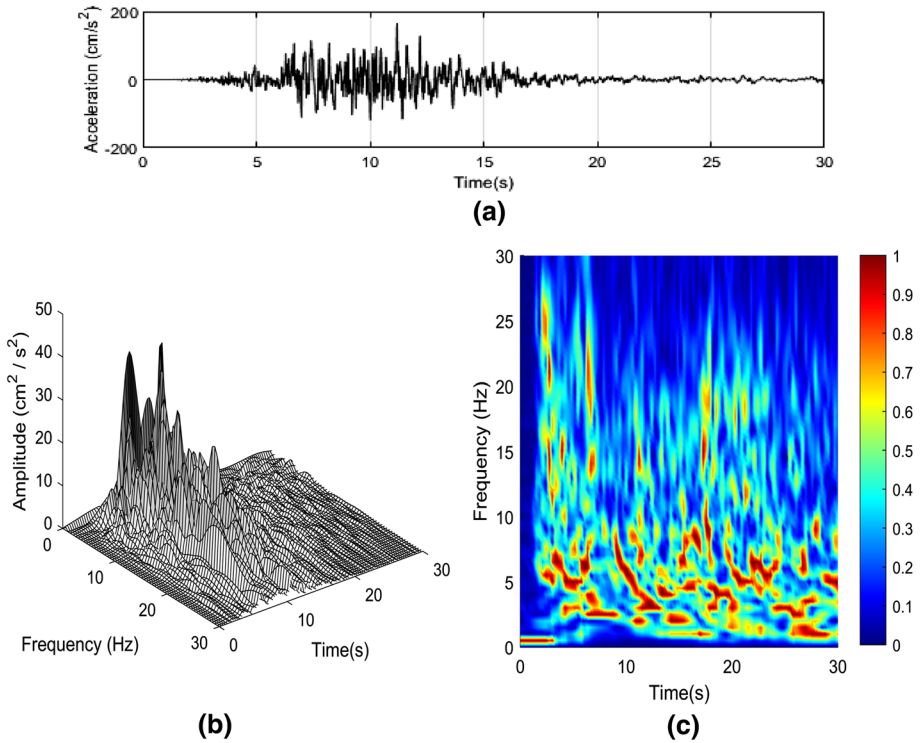


Fig. 1 Application of the S-transform to the EW component of the ground motion recorded at the station of CSC during the central Italy earthquake ($M_w=6.5$, $R_{JB}=12.8$ km, $V_{S30}=698$ m/s) of October 30, 2016: **a** Strong motion record; **b** 3D S-Transform; **c** 2D S-Transform

and substituting PSD with $X_s(t,f)$, the definition of the spectral moments and of the relative parameters becomes:

$$\lambda_i(t) = \int f^i X_s(t,f) df, \quad i = 0, 1, 2;$$

$$Pa(t) = \lambda_0(t)$$

$$F_c(t) = \lambda_1(t)/\lambda_0(t)$$

$$F_b(t) = [\lambda_2(t)/\lambda_0(t) - F_c^2(t)]^{1/2}$$
(4)

where $Pa(t)$, instantaneous average power, is the time envelope function describing the amplitude variation of the ground motion. Its integral in the time domain is equal to the integral of $X_s(t,f)$ in the time–frequency plane and corresponds to the Arias Intensity (Arias 1970, hereafter I_A) in cm^2/s^3 apart from the scale factor $\pi/2$ g:

$$I_A = \int x(t)^2 dt = \iint X_s(t,f) dt df = \int Pa(t) dt$$
(5)

$F_c(t)$, central frequency, and $F_b(t)$, frequency bandwidth, represent the non-stationarity of the frequency content and correspond, respectively, to the centroid of X_s and to the

radius of gyration of X_s with respect to F_c in the frequency plane. With the above-defined parameters, it is possible to derive a lognormal function approximating $X_s(t, f)$:

$$\tilde{X}_s(t, f) = \frac{Pa(t)}{f\sqrt{2\pi\delta}} e^{-[\ln f - \ln \beta(t)]^2 / 2\delta^2} \tag{6}$$

where $\beta(t)$ and δ are derived from $F_c(t)$ and $F_b(t)$ in the following way:

$$\begin{aligned} \ln \beta(t) &= \ln F_c(t) - \delta^2 / 2 \\ \delta &= \sqrt{\ln [1 + F_b^2(t) / F_c^2(t)]} \end{aligned} \tag{7}$$

Pousse et al. (2006) pointed out that a closer analysis of SP96 model shows three basic drawbacks:

1. *Deficit of energy in the low frequency part of the Fourier spectra* of the simulated accelerograms. To overcome this problem, Pousse et al. (2006) proposed to use directly the Brune spectrum (ω -square model) modulated at high frequencies substituting f_{\max} with the central frequency $F_c(t)$. In fact, the adoption of their model, replacing the lognormal distribution in frequency adopted by SP96, does not allow the modulation with time of low frequencies and produces response spectra with unrealistic amplitudes at long periods. Furthermore, as pointed out by Laurendeau et al. (2012), the ω -square point-source model leads to an overestimation of the spectral amplitude at long periods for large magnitude events. Atkinson and Silva (2000) have shown that, despite its success in modeling high ground motions, the single-corner-frequency point source model overpredicts ground motions from moderate-to-large earthquakes at low-to-intermediate frequencies (0.1 to 2 Hz). To overcome the effective lack of low frequencies in SP96 model, generating lower values than expected in the simulated velocity and displacement time series, we adopted a modified function to approximate $\tilde{X}_s(t, f)$ in the frequency domain.

This function is equal to the lognormal function reported in Eq. 6 for $f > f_m$ and to the arithmetic mean of the lognormal function $\tilde{X}_s(t, f)$ and the ω -square Brune spectrum $\Omega(f)$ for $f < f_m$, where f_m is the frequency corresponding to the maximum value of the lognormal function. The Brune ω -square spectrum is defined as (Boore 2003; Pousse 2006):

$$\Omega(f) = \frac{(2\pi f)^2}{1 + (f/f_c)^2} \tag{8}$$

$$\log_{10} f_c = 1.341 + \log_{10} (\beta (\Delta\sigma^{1/3})) - 0.5M \tag{9}$$

where f_c is the corner frequency, $\Delta\sigma$ is the stress drop in bars, M is the moment magnitude, and β is the shear-wave velocity in kilometers per second. The adopted values are $\Delta\sigma = 50$ bars and $\beta = 3.5$ km/s (Bindi and Kotha 2020).

The modified function $\tilde{X}'_s(t, f)$ is thus equal to:

$$\tilde{X}'_s(t, f) = \begin{cases} \text{mean} [\Omega(f), \tilde{X}_s(t, f)] & \text{if } f \leq f_m \\ \tilde{X}_s(t, f) & \text{if } f > f_m \end{cases} \tag{10}$$

Figure 2 shows the fit of the spectral densities corresponding to the record of Fig. 1 at time $t=5$ s, with the modified lognormal described above and derived from the spectral moments of the corresponding time history (Eq. 4). It is evident the increase in low frequency content respect to the former model of SP96 and the good fit of the modified lognormal with the PSD of the real record.

2. *Envelope of simulated accelerograms.* SP96 used a single-time envelope, $Pa(t)$ in Eq. (4), for the whole time series. Pousse et al. (2006) considered, besides S waves, also P and coda waves. Even if this modification does not significantly change the energy distribution of the simulated accelerograms (in their model P waves amplitude is assumed 1/25 of S waves amplitude) we included their suggestion in our new model.
3. *Lack of variability in the simulated ground motion.* Regarding the third problem detected by Pousse et al. in the original SP96 model, it is shown (Sect. 5—Table 3) that the difference among the simulated accelerograms given only by the introduction of the random phases, produces a variability, calculated as $(\max-\min)/\min$, ranging from 60 to 150% in the Arias intensity and peak values. The coefficient of variation ranges between 7 and 19%, showing a lower variability respect to the natural accelerograms. However, we did not include further variability, e.g. considering the standard deviation of the values predicted by the GMM, in order to avoid an excessive dispersion of the results and to have a good match with the target response spectrum even for a small number of simulations. We considered a variability between 0 and 1σ of the values foreseen by the empirical predictive equations only for the strong-motion duration that is not affected by the phase variability.

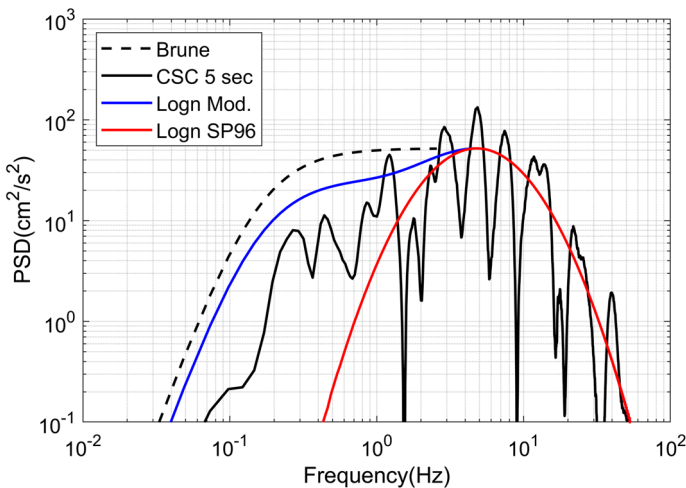


Fig. 2 Fit of the PSD (squared Fourier transform of acceleration) corresponding to the record of Fig. 1 at time $t=5$ s. with the original lognormal function adopted by SP96 (red line) and the modified function (Eq. 10) used in this work (blue line). For comparison, the normalized square of the Brune model calculated with the Eqs. (18) and (19) for $M_w=6.5$ is also shown (dotted line)

3 Ground-motion predictive models

3.1 PGA, PGV, acceleration response spectra, Arias intensity and Vanmarcke duration

A Ground Motion Model (GMM) for Italy has been recently calibrated by Lanzano et al. (2019), hereafter ITA18, with the aim of updating the existing GMMs for shallow active crustal regions in Italy (Bindi et al. 2011). The calibration dataset includes 5607 records, 146 events, and 1657 stations, from Italian and few well-sampled worldwide earthquakes in the magnitude range 3.5–8.0. The GMMs have been derived for the horizontal component (i.e. RotD50; Boore 2010) of Peak Ground Acceleration (PGA), Peak Ground Velocity (PGV) and of 36 ordinates of acceleration response spectra (SA) at 5% damping in the period range 0.01–10 s.

The functional form adopted by Lanzano et al (2019) is:

$$\log_{10}Y = a + F_M(M_w, SOF) + F_D(M_w, R) + F_S(V_{S,30}) + \varepsilon \tag{11}$$

The source term, F_M , the distance term, F_D , and the site-effect term, F_S , are:

$$F_M(M_w) = f_j SOF_j + b_1(M_w - M_h) \text{ if } M_w \leq M_h; b_2(M_w - M_h) \text{ if } M_w > M_h \tag{12}$$

$$F_D(M_w, R) = [c_1(M_w - M_{ref}) + c_2] \log_{10} \sqrt{R_{JB}^2 + h^2} + c_3 \sqrt{R_{JB}^2 + h^2} \tag{13}$$

$$F_S(V_{S,30}) = k \log_{10} \left(\frac{V_0}{800} \right) \tag{14}$$

$$\text{Where } V_0 = \begin{cases} V_{S,30} & \text{for } V_{S,30} \leq 1500 \frac{m}{s} \\ 1500 \frac{m}{s} & \text{otherwise} \end{cases} \tag{15}$$

$a, b_1, b_2, c_1, c_2, c_3, f_j$ and k are the fixed coefficients obtained by a mixed-effects regression (Bates et al. 2015). The hinge M_h and the reference M_{ref} magnitudes and the pseudo-depth h are pre-computed from a first stage nonlinear regression. The distance measure adopted is the closest horizontal distance to the vertical projection of the rupture as defined by Joyner and Boore, hereafter R_{JB} (Abrahamson and Shedlock 1997). SOF_j are the dummy variables for the Style Of Faulting (SOF) and the associated coefficients f_j provide such correction. To discriminate among different focal mechanisms, we set $j=1$ for strike-slip, $j=2$ for reverse, and $j=3$ for normal fault types. The coefficient for normal faulting is constrained to zero to perform the regression ($f_3=0$).

The symbol ε in Eq. (11) represents the total residual associated with the median prediction. ε is decomposed in between-event, site-to-site, and event-and site corrected residuals (Atik et al. 2010), with standard deviation τ, ϕ_{S2S} and ϕ_0 , respectively. The total standard deviation σ can be computed as:

$$\sigma = \sqrt{\tau^2 + \phi_{S2S}^2 + \phi_0^2} \tag{16}$$

In the paper by Lanzano et al. (2019), the period-dependent hinge magnitude M_h in Eq. (12) is assumed as 5.5 at short periods in the range $T=0.01-0.4$ s, $M_h=5.8$ at

intermediate to long periods ($T=0.45-5$ s.) and $M_h=6.3$ at periods longer than 5 s. However, the step between $T=0.4$ s and 0.45 s causes unsmoothed shape of the predicted acceleration response spectra of events of moment magnitude around 6.0. To overcome such problem, we smooth the M_h bump from 5.5 to 5.8, using a stepwise variation of the hinge magnitude in a broader range of periods from $T=0.25$ s to 0.7 s. The table of the re-calibrated coefficients and standard deviations is provided in the appendix.

The dataset used to derive ITA18 is also employed to calibrate the empirical equations to predict the horizontal component (RotD50) of the Arias Intensity computed as:

$$I_A [\text{cm/s}] = \frac{\pi}{2g} \int_0^{T_d} a^2(t) dt \tag{17}$$

The functional form of I_A is the same as Eq. (11), except for the source function F_M , which is modified as:

$$F_M(M_w) = f_j SOF_j + b_{11} M_w + b_{21} M_w^2 \tag{18}$$

We decided to adopt the quadratic scaling rather than the bi-linear, since it provides a better fit with data in this case. The coefficients of I_A predictive equation and the associated standard deviations are shown in Table 1, in which some statistical indexes to verify the goodness of the fit are also reported. In addition to the standard error (SE), the p -value (Wasserstein and Lazar 2016) of the fixed coefficients is also included. Since only a coefficient with a p -value < 0.05 can be considered a meaningful contribution to the model, the correction for style of faulting (coefficients f_1 and f_2), shows a limited impact on predictions.

The total standard deviation associated to the prediction is 0.574, which is in line with the findings of Foulser-Piggot and Goda (2015), for Japanese data, and Sandikkaya and Akkar (2017), for European data, that are 0.596 and 0.560, respectively. The standard

Table 1 Calibration parameters and statistical indexes of the regression for I_A . SE= standard error

Calibration parameters	Arias intensity, I_A (cm/s)		
	Value	p -Value	SE
a	- 2.2907	0.0006	0.6713
b_{11}	1.4033	1.339E-08	0.2467
b_{21}	- 0.0881	0.0001	0.0227
c_1	0.4870	2.443E-186	0.0161
c_2	- 1.0667	2.699E-90	0.0520
c_3	- 0.0054	6.706E-75	0.0003
k	- 1.0309	9.135E-66	0.0594
f_1	0.1185	0.0442	0.0589
f_2	- 0.0176	0.7564	0.0569
M_{ref}	7.5	-	-
h (km)	5.0	-	-
τ	0.259		
ϕ_{S2S}	0.393		
ϕ_0	0.328		
σ	0.574		

deviation is larger than the sigma obtained by SP96 (0.397), since their empirical relation was calibrated on a small number of observations compared to ITA18 dataset.

Figure 3 shows a comparison between the I_A calibrated in this study and that estimated by SP96. For the sake of comparison, the latter has been scaled by a coefficient of 0.86 (Sabetta 2003) to convert the SP96 predictions, which represent the maximum between horizontal components, into RotD50. The ITA18 predictions are lower than SP96, and the difference increases with increasing source to site distance, a trend which has been already observed for other strong-motion parameters (e.g. PGA, PGV) and attributed to the limited data set of analog data used to derive SP96. To predict the significative duration of the ground motion, we did not use the definition of Trifunac and Brady (1975), corresponding to the interval between the achievement of 5 and 95% of the Arias intensity, because it tends to overestimate the duration in case of multiple shock recordings and it is not always well correlated to the strong part of the accelerogram. We preferred the DV definition given by Vanmarcke and Lai (1980) in its simplified formulation $DV = 7.5I_A/PGA^2$.

This kind of duration is proportional to the ratio I_A/PGA^2 and is well correlated to the strong phase of the motion corresponding to the arrival of S waves.

Following Bommer et al. (2009), the empirical equations for the ground motion duration are calibrated using the same functional form of Eq. (8). The calibration results are given in Table 2. The p -values of the style of faulting coefficients are still the highest, confirming that the introduction of this explanatory variable in the model has a negligible impact on the standard deviation (Bommer et al. 2003; Lanzano et al. 2019). The total standard deviation (0.211) is comparable to that obtained by SP96 (0.247).

Figure 4 shows a comparison between DV predictions of ITA18 (this study) and SP96. The predictions of the two models are similar in near source conditions, while as the distance increases, ITA18 grows more than SP96, especially for low magnitude events.

3.2 Central frequency $F_c(t)$ and frequency bandwidth $F_b(t)$

Besides I_A and DV, two further parameters must be calibrated over the data, i.e. the central frequency, $F_c(t)$, and the frequency bandwidth, $F_b(t)$ (Eq. 4).

Fig. 3 I_A prediction obtained in this study compared with SP96 for rock sites ($V_{S30}=800$ m/s), normal fault and two magnitudes (4.5 and 6.5). Observations are from the ITA18 database (Lanzano et al. 2019). The dashed thin lines represent the range corresponding to ± 1 standard deviation of the proposed model

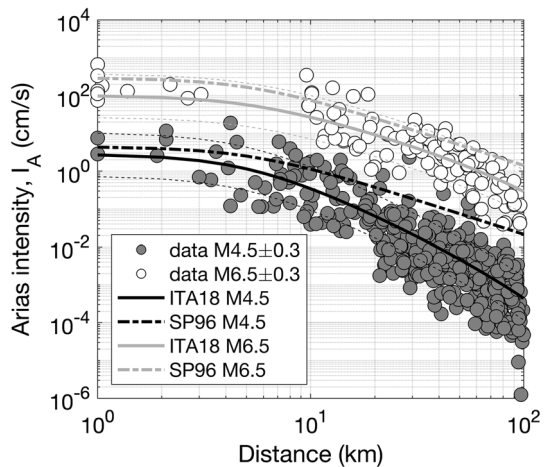
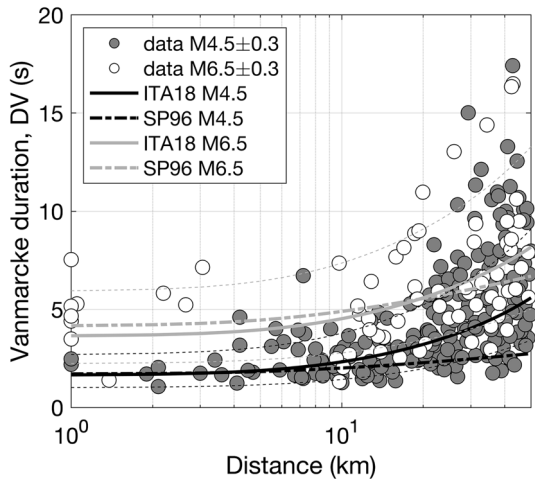


Table 2 Calibration parameters and statistical indexes of the regression for Vanmarcke duration DV. SE = standard error

Calibration parameters	Vanmarcke duration, DV (s)		
	Value	<i>p</i> -Value	SE
<i>a</i>	0.434	8.656E-32	0.0368
<i>b</i> ₁	0.249	6.939E-62	0.0148
<i>b</i> ₂	0.495	1.598E-09	0.0819
<i>c</i> ₁	-0.098	1.208E-44	0.0069
<i>c</i> ₂	0.258	5.256E-34	0.0211
<i>c</i> ₃	0.002	2.876E-36	0.0001
<i>k</i>	-0.252	1.03E-30	0.0217
<i>f</i> ₁	-0.05	0.007201	0.0188
<i>f</i> ₂	-0.032	0.07731	0.0181
<i>M</i> _h	7.0	-	-
<i>M</i> _{ref}	7.0	-	-
<i>h</i> (km)	6.0	-	-
<i>τ</i>	0.078	-	-
<i>φ</i> _{S2S}	0.133	-	-
<i>φ</i> ₀	0.145	-	-
<i>σ</i>	0.211	-	-

Fig. 4 Comparison between the prediction of SP96 and this study (ITA18) for rock sites (*V*_{S30} = 800 m/s), normal fault and two magnitudes (4.5 and 6.5). The dashed thin lines represent the range corresponding to ± 1 standard deviation of the proposed model



To better constrain the predictive models and control the associated variability, ITA18 data are further skimmed considering a distance from the source lower than 50 km and a PGA greater than 0.03 g. This was done to eliminate noisy recordings with small amplitudes and long duration. Furthermore, the records that include secondary events have been excluded, since they produce anomalous trends of *F_c* and *F_b* with time. Despite the additional selection leads to the loss of more than 80% respect to the initial dataset (953 records vs 5607), the data still guarantee a good sampling in terms of *M_w*, *R_{JB}* and *V_{S30}* as can be appreciated in Fig. 5.

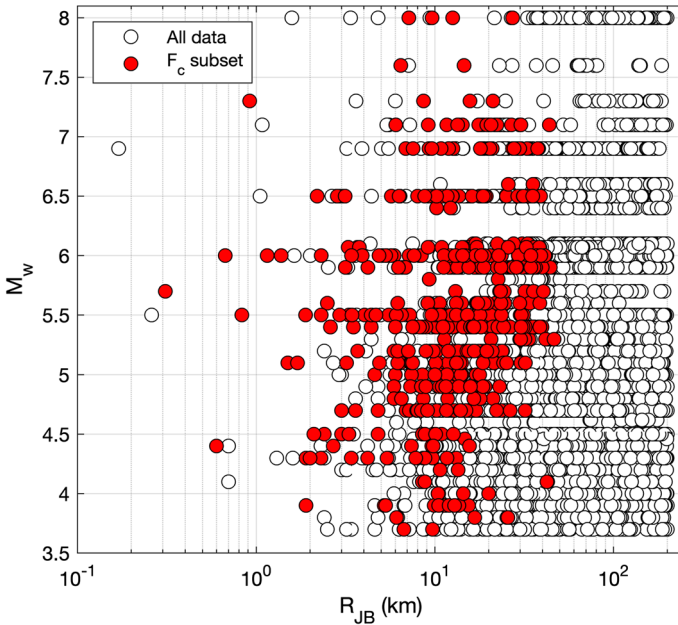


Fig. 5 Magnitude-Distance distribution of the data used in this study: white dots (5607 data) refer to the I_A and DV calibration; red dots (953 data) refer to F_c and F_b calibration

Unlike I_A and DV, F_c and F_b are extremely sensitive to the selected signal length as shown in Laurendeau (2013). Therefore, the accelerometric traces have been shortened in time using different percentages (5–95%, or 10–90%) of the Arias intensity. The first cut allows the signal to start approximately at the arrival of the P waves, while the second cut removes the tail of the signal that could contain surface waves or secondary events. The cut traces were then decimated to reduce the time sampling from 0.005 to 0.015 s. The S-transform of the signals was computed according to Eq. (1) and the fitting lognormal parameters according to Eq. (4).

Once $F_c(t)$ and $F_b(t)$ have been calculated for all the selected recordings, we got a dataset of more than 340,000 observations. Figure 6 provides the trend of $\log_{10}(F_c)$ and F_b/F_c , which are the parameters required for the calculation of the lognormal function in Eq. (6), as a function of time, magnitude and V_{S30} . Considering the scarce contribution of $t > 45$ s. for the simulation of waveforms, the data were furtherly cut out in time before the climb back up of F_c around 45 s shown in Fig. 6a.

The data are affected by large uncertainties, but the median trends are comparable to those predicted by SP96: $\log_{10}(F_c)$ decreases with time and magnitude, while increases with rising V_{S30} . The time decay of F_b and F_c is similar, so that their ratio can be considered time independent, while a weak dependence on magnitude and V_{S30} is observed.

Laurendeau (2013) and Hong and Cui (2020) also showed that F_b and F_c data are affected by a large dispersion in the case of Japanese and NGA-West2 records, respectively, so that the predicting equations as a function of M_w , R_{JB} and V_{S30} suffer relevant uncertainties. For this reason, we performed several trial calibrations to define the optimal expressions. Figure 7 shows the behavior of F_c versus time obtained for different time windows, based on different fractions of Arias Intensity and length of the signal. Among the models

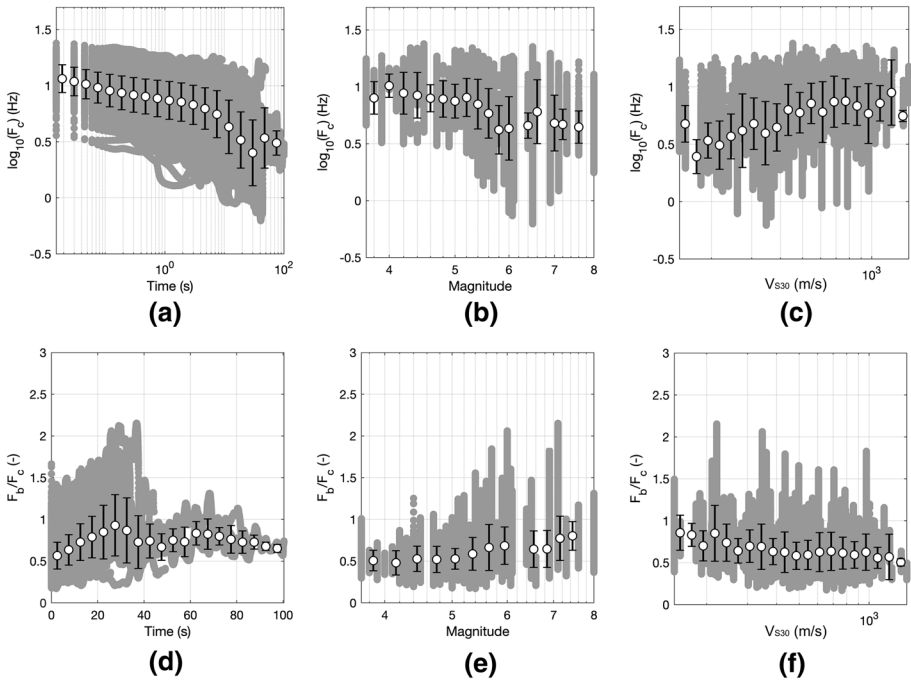


Fig. 6 Distribution of the parameters required for the calculation of the lognormal function: **a–c** = $\log_{10}(F_c)$ as a function of **a** time, **b** moment magnitude and **c** V_{S30} ; F_b/F_c as a function of **a** time, **b** moment magnitude and **c** V_{S30}

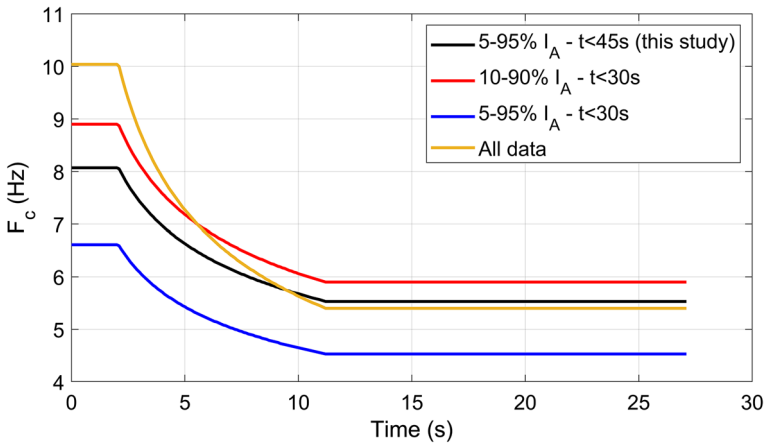


Fig. 7 Behavior of F_c versus time according to Eq. 16 and different I_A and time interval selection in case of $R_{jb}=20$ km, $V_{S30}=800$ m/s and $M_w=6$. In this study the black curve was adopted

illustrated in the figure, we adopted the one giving the best fit between the response spectra obtained with the simulations and those predicted by ITA18 (black line, that is intermediate between different models and providing the highest coefficient of determination R^2).

The distance dependence of $F_c(t)$ is taken into account shifting the F_c curve in time, according to the focal distance. F_c begins to decrease at time T_p when P waves arrive and remains constant after time T_{coda} , denoting the arrival of the coda waves (T_p and T_{coda} are defined in the following paragraph). Due to the large uncertainties in the F_c and F_b regressions and the weakness discussed previously of the statistical significance of the style of faulting coefficients for I_A , and DV, we did not include a SOF coefficient in F_b and F_c predictive equations. The SOF dependence in the simulated time series is given by the coefficients included in I_A , and DV regressions. The final predictive equations are the following:

$$\ln[F_c(t)] = 3.5 - 0.224\ln(t) - 0.208Mw + 0.42\ln(V_{S,30}/800) \tag{19}$$

where $t = T_p$ if $t < T_p$; $t = T_{coda}$ if $t > T_{coda}$

$$F_b/F_c = 0.44 + 0.07Mw - 0.1\ln(V_{S,30}/800) \tag{20}$$

4 Time series simulation

After the evaluation of $Pa(t, M_w, R_{JB}, V_{s30}, SOF)$, $F_c(t, M_w, V_{s30})$, and $F_b/F_c(M_w, V_{s30})$, it is possible, for a given magnitude M_w , source-to-site distance (R_{JB} or R_{epi}), site condition V_{S30} , and style of faulting SOF, to calculate an approximated $\tilde{X}_s(t, f)$ from Eq. (10).

The amplitude of $\tilde{X}_s(t, f)$ in the frequency domain is approximated by a modified log-normal function defined by Eqs. (6), (7), and (10).

The amplitude of \tilde{X}_s in the time domain, $Pa(t)$, represents the time envelope of the simulated accelerograms and, following Pousse et al. (2006), is defined by two lognormal followed by an exponential decay. In this way are considered the arrival time, energy, and broadening of the P and S pulses with distance, as well as the existence of scattered waves that produce the coda of the accelerogram. The shape of the P and S pulses is described by lognormal distributions truncated at time $t = T_{coda}$ and is prolonged by an algebro-exponential function simulating the coda waves according to the following formula:

$$Pa(t) = \frac{1}{25} \frac{I_A}{t\sqrt{2\pi\sigma_p}} \exp\left(-\frac{1}{2\sigma_p^2}(\log(t) - \mu_p)^2\right) + \frac{24I_A}{25} \times \begin{cases} \frac{1}{t\sqrt{2\pi\sigma_s}} \exp\left(-\frac{1}{2\sigma_s^2}(\log(t) - \mu_s)^2\right) & t \leq T_{coda} \\ A_0 \cdot t^{-\alpha} \cdot e^{-\frac{\omega t}{Qc}} & t \geq T_{coda} \end{cases} \tag{21}$$

In Eq. (21), the first term on the right-hand side corresponds to the P wave, the second represents the S wave, and the last one corresponds to the coda waves. Following Pousse et al. (2006), the coefficients 1/25 and 24/25 are based on the assumption that the body-wave amplitude from a double-couple point source varies in the far field as the velocity to the power of 3 (Lay and Wallace 1995). Since $V_p/V_s = \sqrt{(2 - 2\nu)/(1 - 2\nu)}$, where ν is Poisson's ratio, typically equal to 0.25 for geological materials (Kramer 1996), the S_{wave} amplitude results about five times larger than the P_{wave} amplitude. μ_p , μ_s , σ_p , and σ_s are the expected mean values and standard deviations of the distribution of the parameter t for the P and S pulses, respectively. The parameters μ_p and μ_s control the time at which the maximum amplitudes of the P and S pulses are reached. Moreover,

σ_p and σ_s characterize the broadening of the P and S pulses. For $t \geq T_{\text{coda}}$, Eq. (21) represents the coda waves decay (Aki and Chouet 1975). A_0 is the source term representing the effect of both P and S waves; α is a parameter equal to 2 for body waves; Q_c is the coda Q-value, assumed equal to Q_s under the hypothesis of a common wave attenuation mechanism for the S and the coda wave (Roecker et al. 1982). We adopted $Q_s = 250 \cdot f^{0.29}$ (Bindi and Kotha 2020). Figure 8 shows the graphical definition of the envelope function $Pa(t)$ proposed in this study in comparison with that adopted in SP96.

The time value indicated with T_{sp} corresponds to the time delay in seconds between S and P waves arrival and is calculated by dividing the focal distance R_{hypo} in kilometers by the factor $V_p \times V_s / (V_p - V_s)$ assumed to be equal to 7 km/s. R_{hypo} is calculated, for a depth given as input parameter, from the epicentral distance in turn obtained from R_{JB} according to the procedure suggested by Scherbaum et al. (2004). The choices of T_p , T_s , T_{coda} , σ_p , σ_s , were derived from several tests with real accelerograms to have a time envelope function $Pa(t)$ with the following characteristics:

- a modal value, at time $t = T_s$, correlated to the focal distance;
- a standard deviation proportional to the strong-motion duration DV;
- an area equal to the Arias intensity I_A ;
- a total duration 30% greater than the value corresponding to the modal value plus 3DV.

Figure 9 shows, for a simulated accelerogram, the corresponding time envelope function $Pa(t)$ and central frequency $F_c(t)$.

The simulation of the accelerograms is then performed summing Fourier series with time-dependent coefficients derived from the approximated square of the S-transform X'_S (Eq. 6) as follows:

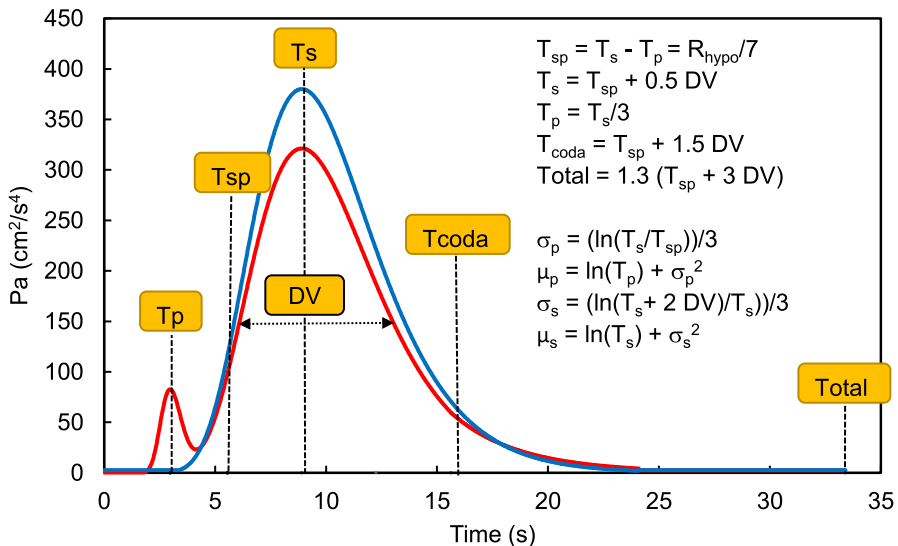


Fig. 8 Graphical definition of the time envelope function proposed in this study (red line) compared with that of SP96 (blue line) for $M_w = 6.5$, $R_{jb} = 30$ km and $V_{S30} = 800$ m/s

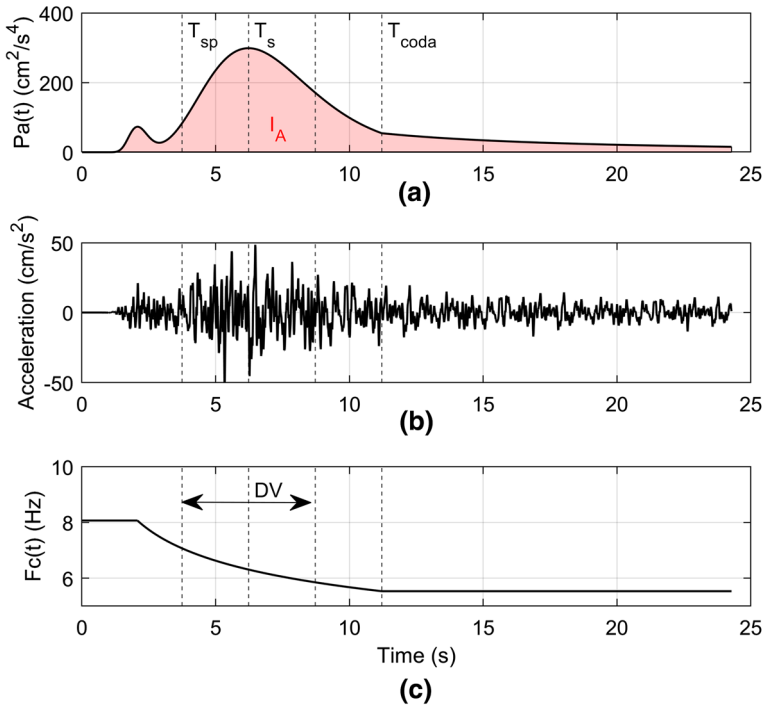


Fig. 9 **a** time envelope function $P_a(t)$ proposed in this study in case of $M_w=6$, $R_{jb}=20$ km and $V_{S30}=800$ m/s; **b** corresponding simulated accelerogram; **c** time behavior of the central frequency $F_c(t)$

$$a(t) = \sum_{n=1}^N C_n(t) \cos(n2\pi f_0 t + \varphi_n) \tag{22}$$

$$C_n(t) = \sqrt{2\pi f_0 \widetilde{X}'_S(f_n, t)} \tag{23}$$

where $a(t)$ is the acceleration, f_0 is the fundamental frequency (reciprocal of the total duration), and the phases φ_n are random numbers uniformly distributed in the range 0 to 2π . The computer code used for the time series simulation has been developed both in Fortran95 and Matlab. As shown in the flow-chart of Fig. 10, it requires as input parameters M_w , distance, focal depth, V_{S30} , and SOF. The choice of the type of distance is constrained by the definition used in the reference GMM that is the fault distance R_{JB} . However, it could be easier for engineers to use, the epicentral distance R_{epi} as basic input parameter. Furthermore, this kind of distance, converted into focal distance, is required to build up the time envelope function $P_a(t)$. Embedded in the code is the magnitude/SOF dependent conversion between R_{epi} and R_{JB} , according to the procedure suggested by Scherbaum et al. (2004). Also included in the code are the computations of I_A , DV , F_c and F_b based on the predictive equations discussed in Sect. 3 and derived from the Italian database. The code produces as output acceleration, velocity, and displacement time series, Fourier spectra, response spectra, and a summary statistics of various measures of ground motion.

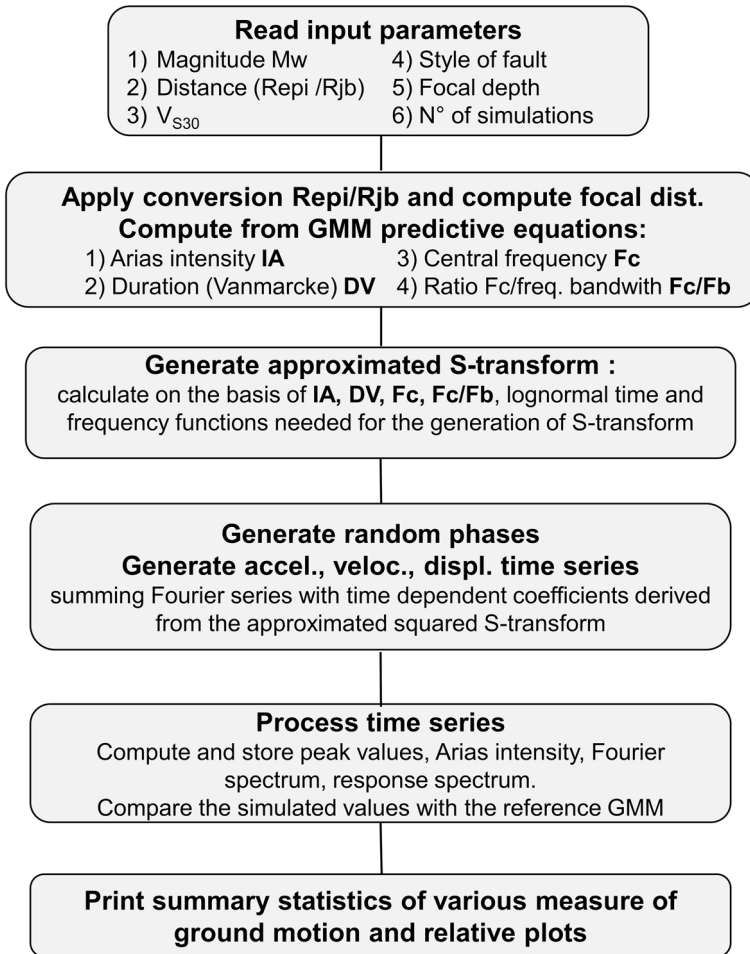


Fig. 10 Flow-chart of the computer code used for the time series simulation

5 Results

Figure 11 shows nine simulated accelerograms in case of normal fault, $V_{S30}=800$ m/s, $R_{JB}=10$ km, and $M_w=5, 6,$ and 7 . For each value of magnitude, three different simulations, differing only in the phase values, are plotted. The increase of magnitude causes an increase of amplitude, duration, and low-frequency content of the signal. The PGA of the simulated signals, reported on each time history, is close to the values predicted by the ITA18 GMM, that are equal to 46, 115 and 224 cm/s^2 for the three values of magnitude, respectively. The attenuation of the simulated accelerograms with the distance from the source is represented in Fig. 12, in case of strike-slip fault, $V_{S30}=400$ m/s, $M_w=6,$ and $R_{JB}=10, 30,$ and 50 km, respectively. As expected, larger distances correspond to smaller accelerations and longer durations.

Figure 13 shows the effect of different site conditions on the simulated time series of acceleration and velocity for normal fault, $M_w=6.5,$ $R_{JB}=10$ km, and V_{S30} equal to 400,

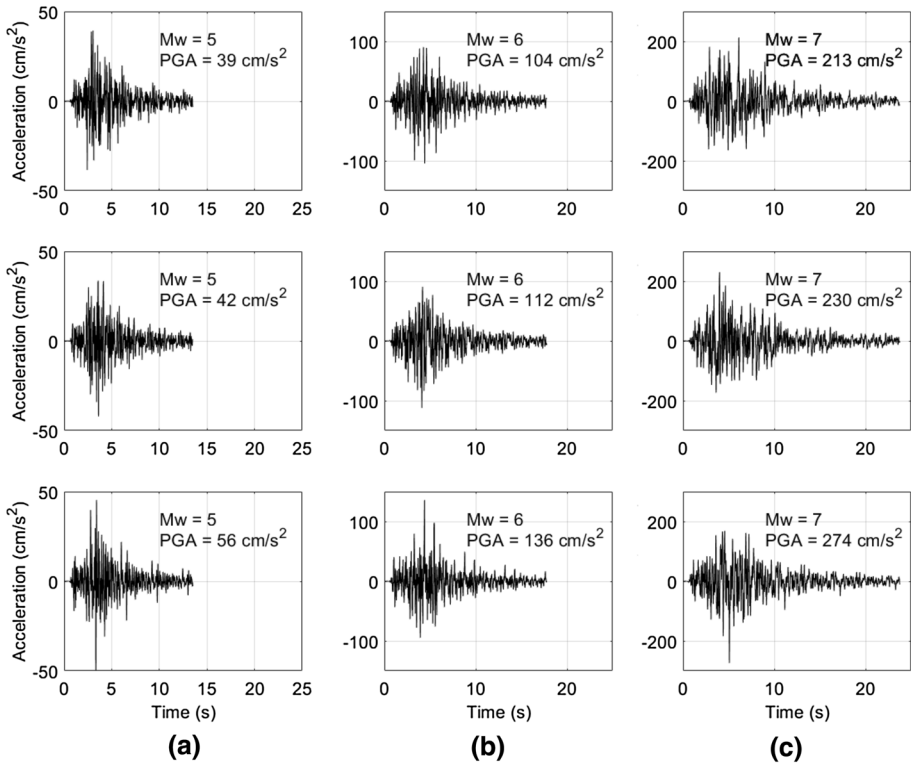


Fig. 11 Simulated accelerograms in case of normal fault, $V_{S30}=800$ m/s, $R_{JB}=10$ km, and **a** Mw=5, **b** Mw=6 and **c** Mw=7. For each value of magnitude, three different simulations (a,b,c), differing only in the phase values, are plotted

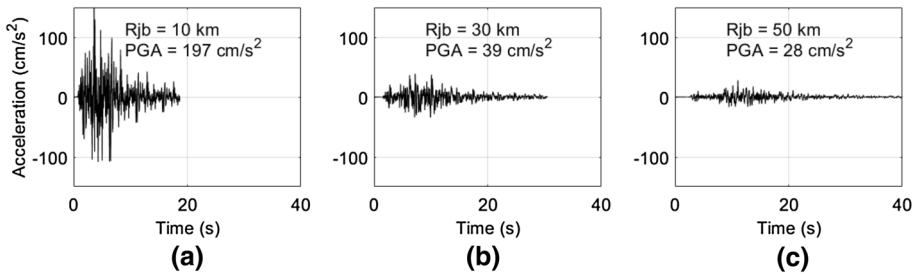


Fig. 12 Simulated accelerograms in case of strike-slip fault, $V_{S30}=400$ m/s, $M=6$, and R_{JB} equal to: **a** 10, **b** 30, and **c** 50 km

600, and 800 m/s, respectively. Going from soft to stiff soil, it is observed a decreased content of low frequencies, lower peak values, and a decrease in the duration of the time series.

Figure 14 shows a comparison of the accelerogram recorded at the AQP station during an aftershock of the April 2009 L'Aquila earthquake ($M_w=5.4$, $R_{JB}=8.9$ km, normal fault, $V_{S30}=836$ m/s) with five different simulations in terms of acceleration. Figure 15 shows the same comparison in terms of velocity, displacement and Fourier spectrum.

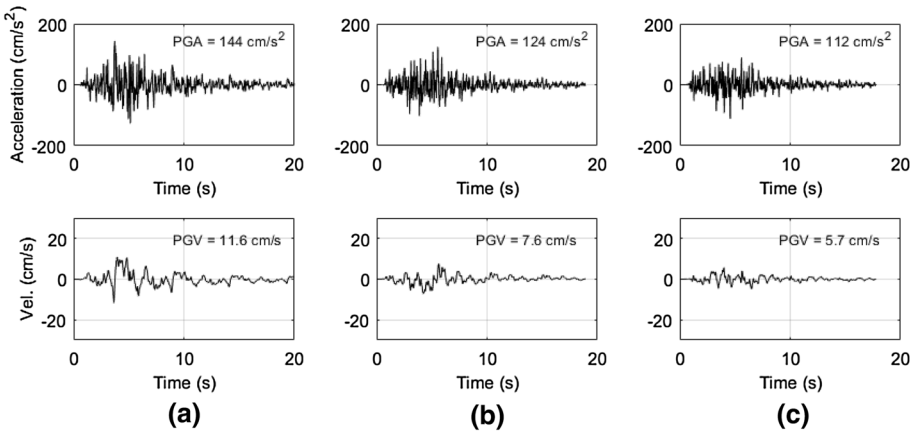


Fig. 13 Simulated time series of acceleration and velocity for $M_w=6$, $R_{JB}=10$ km, normal fault, and V_{S30} respectively equal to **a** 400, **b** 600, and **c** 800 m/s

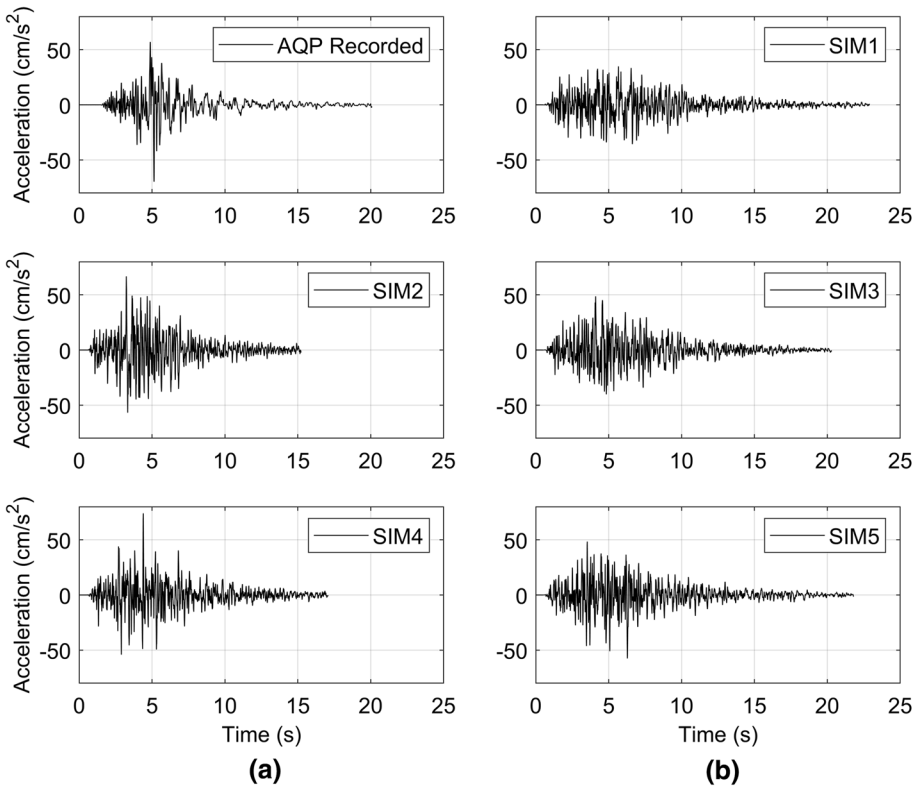


Fig. 14 Accelerogram recorded at AQP station during the L'Aquila earthquake ($M_w=5.4$, $R_{JB}=8.9$ km, normal fault, $V_{S30}=836$ m/s) of April 2009, compared with five different simulations in terms of acceleration

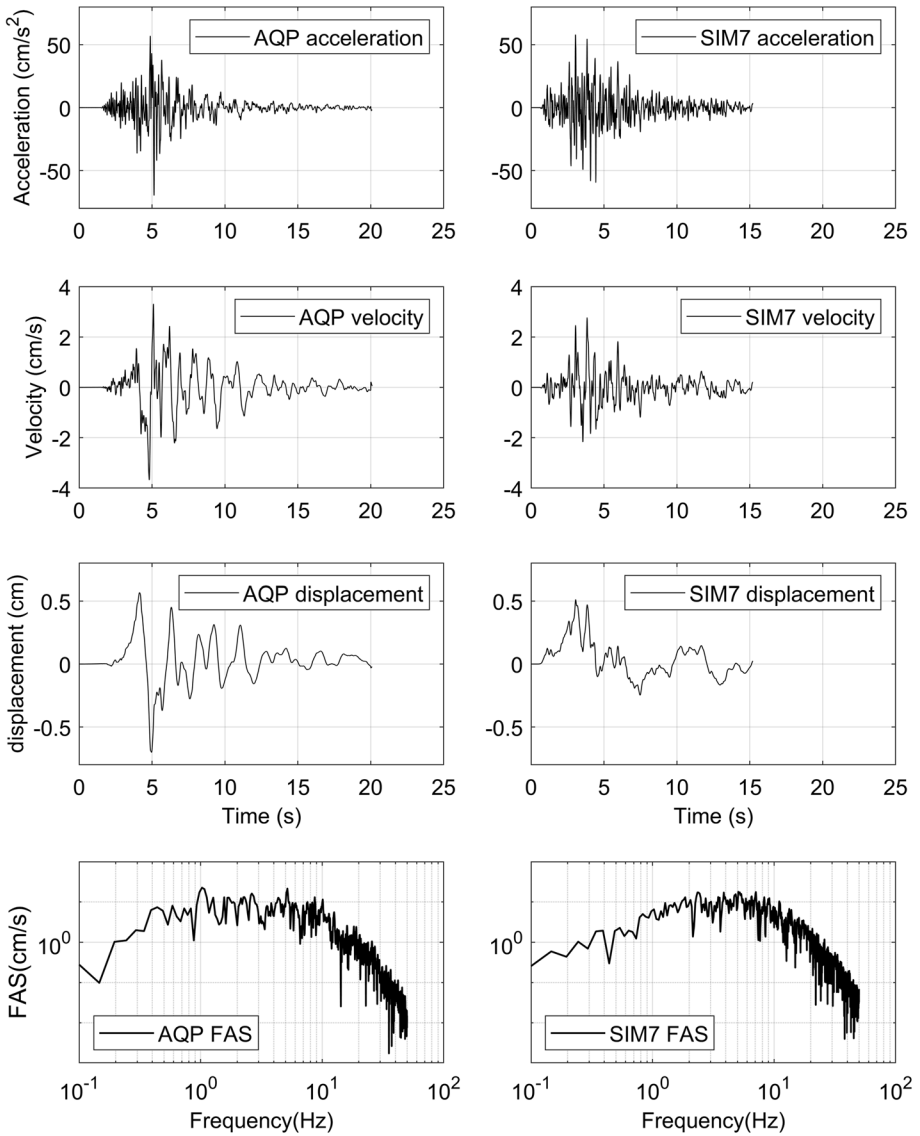


Fig. 15 Accelerogram recorded at AQP station during the L’Aquila earthquake of April 2009 compared with a single simulation in terms of velocity, displacement, and Fourier spectra

The variability included in the simulated time-series only by the introduction of the random phases, is further illustrated in Table 3 for several scenarios in terms of magnitude distance and site conditions. The percentage variability of 100 simulations, calculated as $(\max - \min) / \min$ is around 60% for Arias Intensity, 120% for PGA, and 150% for PGV. The coefficient of variation is instead around 9% for Arias, 16% for PGA, and 17% for PGV. As discussed in Sect. 2, we assume this kind of variability as suitable for engineering applications. The value predicted by ITA18 is always between the mean

Table 3 Arias intensity, PGA, and PGV resulting from 100 simulations for different scenarios compared with the values predicted by the GMM of Lanzano et al. (ITA18)

	$M_W=5$ $R_{JB}=10$ km $V_{S30}=400$ m/s	$M_W=6.5$ $R_{JB}=30$ km $V_{S30}=800$ m/s	$M_W=6.0$ $R_{JB}=50$ km $V_{S30}=600$ m/s	$M_W=7$ $R_{JB}=5$ km $V_{S30}=800$ m/s
<i>ARIAS (cm²/s³)</i>				
Mean	1485	2710	354	71,799
CV (%)	11.0	8.9	7.3	8.7
Min	1147	2129	271	54,609
Max	1900	3265	443	86,159
Var (%)	66	53	63	58
ITA18	1493	2728	358	72,477
<i>PGA (cm/s²)</i>				
Mean	49.7	50.7	16.5	306.1
CV (%)	19.3	16.6	13.3	15.3
Min	34.8	37.8	11.6	219.4
Max	79.1	88.1	22.3	466.6
Var (%)	127	134	92	113
ITA18	60.4	52.4	17.7	317.8
<i>PGV (cm/s)</i>				
Mean	2.6	4	1.4	25.3
CV (%)	19.2	17.5	14.3	18.2
Min	1.6	2.7	0.8	15.2
Max	4.6	6.7	2	37.9
Var (%)	188	148	150	149
ITA18	2.8	4.3	1.4	28.5

CV (%) is the coefficient of variation in percentage. Var (%) is the variability calculated as 100*(max-min)/min

value ± standard deviation of the simulations. Considering that the phase variation only acts on the peak values and that the selected definition of duration gives rather short durations compared to real records, we chose to add a uniform variation of DV between 0 and 1 standard deviation given by the regression equation. The effect of the DV variation is shown in the plots of Fig. 14.

Figure 16 shows a comparison of the Fourier spectra obtained from our model (mean of seven simulations) and the spectra derived from the Brune ω-square model defined in Eq. (8) and (9) with the addition of the high frequency cutoff and the geometrical and anelastic attenuation (Boore 1983) as:

$$A(f) = CM_0 \frac{(2\pi f)^2}{1 + (f/f_c)^2} e^{-\pi k f} \cdot \frac{e^{-\pi f R/Q\beta}}{R} \tag{24}$$

C is a constant given by

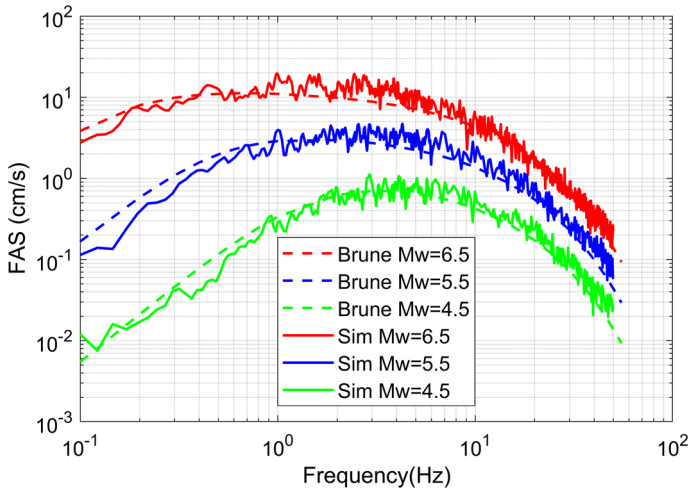


Fig. 16 Comparison among the Fourier Amplitude Spectra (FAS) of the mean of 7 simulated accelerograms and the spectra derived from the seismological ω -square Brune model for magnitudes 4.5, 5.5, 6.5, $R_{JB}=30$ km and $V_{S30}=800$ m/s

$$C = \frac{F_S R_{\theta,\varphi} P}{4\pi \rho \beta^3} \tag{25}$$

where $F_S=2$ is the free surface correction, $R_{\theta,\varphi}=0.63$ is the average radiation pattern, and $P=1/\sqrt{2}$ is the partition between the two horizontal components; $\rho=2.7$ g/cm³ and $\beta=3.5$ km/s are the density and S-wave velocity in the crust. M_0 is the seismic moment derived from M_w (Hanks and Kanamori 1979). The corner frequency f_c , defined in Eq. (9), is computed with a stress drop of 50 bars and the parameter k , controlling the high frequency decay, is set equal to 0.018. R is the distance (R_{JB}) and Q is the quality factor, proportional to the frequency as: $Q=Q_0 f^n$, where $Q_0=250$ and $n=0.29$. The values of the parameters adopted for the Brune model have been taken from Bindi et al. (2018) and Bindi and Kotha (2020).

The correspondence of the results obtained with very different models is remarkable, as shown in Fig. 16: the Brune’s ω -square model is theoretical and based, in the frequency domain, on the radiation of the seismic waves from a point source; the model of this study is empirical and based, in the time domain, on the statistical analysis of recorded strong ground motions.

Figure 17 compares the acceleration response spectra of seven simulated accelerograms with the 50-percentile spectrum predicted by ITA18. As comparison criterion we chose, in accordance with the Italian Seismic Code NTC18 (D.M. 2018) and the Eurocode EC8 (CEN 2009), a tolerance range of -10% and $+30\%$ with respect to the target spectrum. The red dotted lines in the figure correspond to the above-mentioned limits respect to ITA18. The black dashed lines represent the bound of \pm one standard deviation of the seven simulations. The level of fit is quite satisfactory even for such a small number of simulations; the variability of the simulated spectra is only due to the different phases of the corresponding time series and does not include the uncertainty in the prediction of the ground-motion parameters used for the simulation. This is exactly

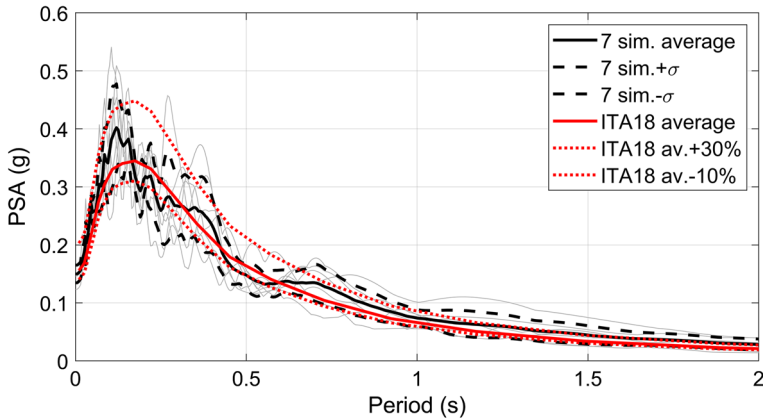


Fig. 17 Response spectra of seven simulated accelerograms compared with the spectrum predicted by ITA18 GMM, in case of normal fault, $M_W=5.5$, $R_{JB}=5$ km, $V_{S30}=600$ m/s;

the aim of this work: provide the engineer with a simulation method that, even with a small number of simulations, matches adequately the target response spectrum.

The averaged response spectra resulting from twenty simulations are compared in Fig. 18 with the spectra predicted by ITA18 for different magnitudes, distances, and site conditions. The fit is satisfactory for all the considered cases.

6 Simulation of time series in engineering applications

The selection of ground motions in the framework of earthquake engineering is becoming increasingly important with the growing use of nonlinear dynamic analyses, for which a set of input ground motions is a key component. The increasing availability of large strong motion databases pushes toward the use of natural accelerograms. However, natural records corresponding to the desired combination of magnitude, distance, soil type and SOF are not always available and appropriate spectral shapes cannot be achieved in case of excessive scaling (Causse et al. 2014). For these reasons, the relatively easy and fast generation of simulated records, compatible with an assigned design spectrum, is still very popular for both practice and research purposes (Iervolino et al. 2010b).

Given the little guidance provided in EC8 on how to select/generate code-compatible time-series, several articles have proposed sets of accelerograms consistent with the standard EC8 spectra (e.g. Giaralis and Spanos 2011; Iervolino et al. 2010b). Iervolino et al. (2010b) evaluated the nonlinear response of three types of SDOF systems applying different methods to generate artificial and simulated time series as well as scaled natural records.

Causse et al. (2014), to assess the variability in the response of engineering systems due to the differences in the simulated input motions, use the ground motion simulation method proposed by Laurendeau et al. (2012), very similar to the methodology proposed here, and compute the nonlinear response of SDOF systems by describing the hysteretic nonlinear behavior with the Takeda model. They show that the non-stationary stochastic approach of generating accelerograms is quite useful in capturing the true ground-motion variability and consequently the variability of building response.

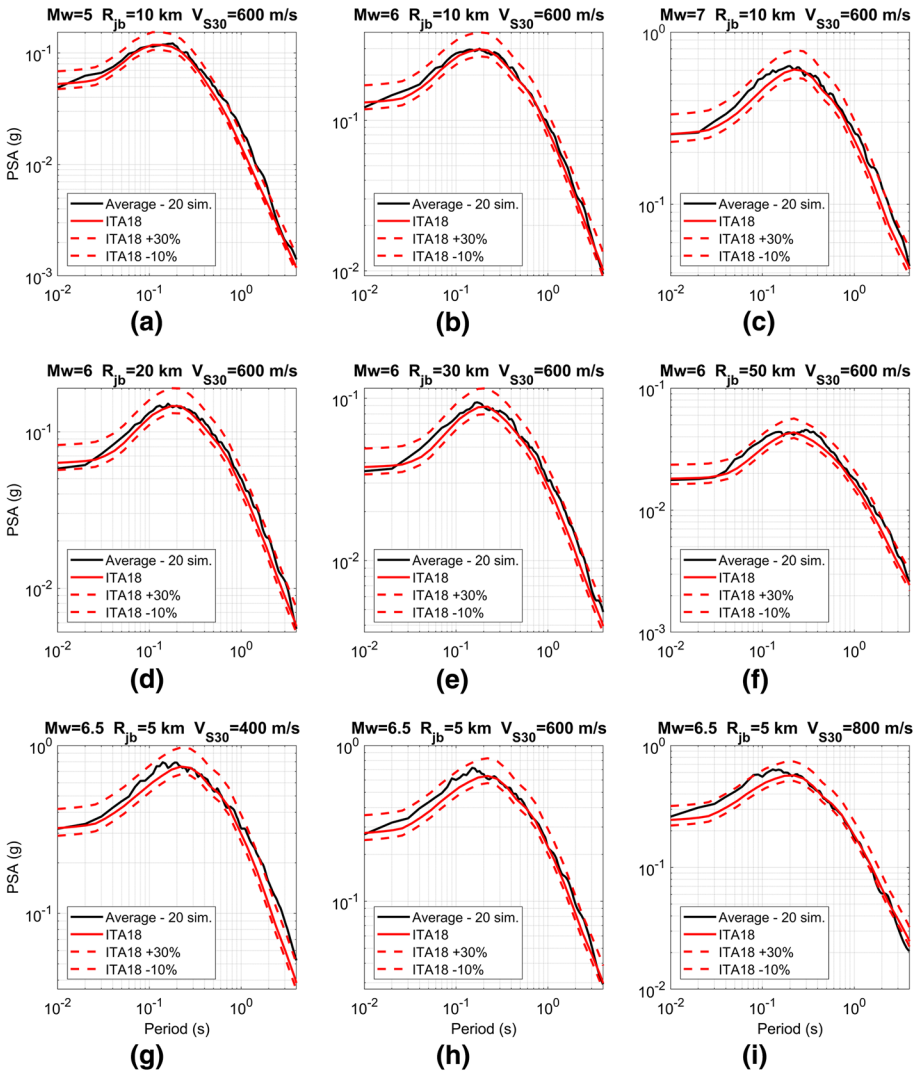


Fig. 18 Comparison of the mean response spectra derived from twenty simulated accelerograms, with the spectra predicted by ITA18 GMM for different: **a, b, c** magnitudes; **d, e, f** distances; **g, h, i** site conditions in case of a normal SOF

To check the compatibility of the response spectra simulated with the method proposed in this study with the provisions of seismic codes, we compared the average spectra of 20 simulated ground motions with the spectral shapes given by the Eurocode EC8 (CEN 2009). Figure 19 shows the good agreement of the simulated spectra, normalized to their PGA, with the spectral shapes foreseen by EC8, type A soil, in case of high (Type 1) and low (Type 2) magnitudes. The Mw values of 7 and 5 correspond to those adopted (Sabetta and Bommer 2002) for the calibration of EC8 type 1 and 2, respectively.

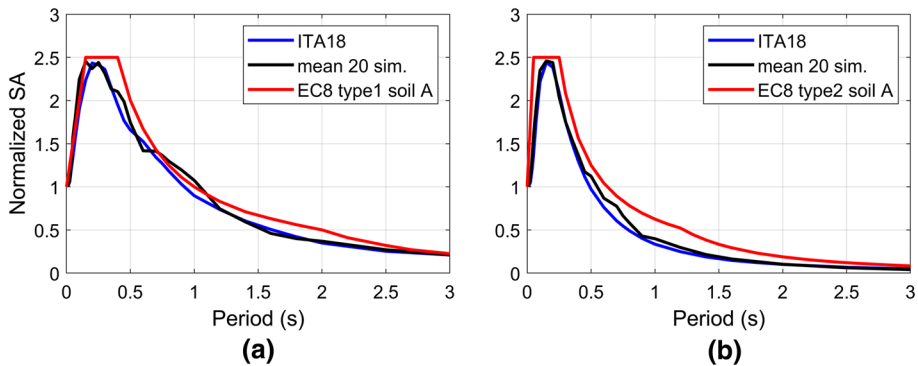


Fig. 19 Comparison of the mean of 20 simulated response spectra (normalized to the PGA) with **a** EC8 type 1 spectrum ($M_w = 7$, $R_{JB} = 5$, $V_{S30} = 800$) and **b** EC8 Type 2 ($M_w = 5$, $R_{JB} = 30$, $V_{S30} = 800$)

7 Conclusions

An improved stochastic model to generate nonstationary acceleration time series has been discussed in this paper. The development of the method is based on the S-transform and, unlike some of the models available in the literature, it considers the modulation both in amplitude and frequency. The amplitude modulation in time is obtained using time envelopes for the P, S, and coda waves. The frequency modulation is achieved with a lognormal function calibrated with the Brune's ω -square model allowing for a realistic frequency content. The simulated time series depend on few input parameters: moment magnitude, source to site distance, shear wave velocity at the site, and style of faulting. The predictive equations for the response spectra are derived from the study of Lanzano et al (2019) with a calibration dataset including 5607 records, 146 events, and 1657 stations, from Italian and few well sampled worldwide earthquakes in the magnitude range 3.5–8.0. The same dataset has been used to derive the predictive equations for the most relevant parameters required for the time series simulation proposed in this study: Arias intensity and significant duration. The predictive equations for central frequency and frequency bandwidth have been derived from a reduced dataset calibrated to exclude records too far from the source, or too small, or including secondary events. The computer code implemented for the simulation, reproduces the non-stationarity in amplitude and frequency of the real ground motions, with promising applications in nonlinear dynamic analysis of structures. The simulated time series statistically fit the prediction of the ITA18 GMM for several ground-motion amplitude measures, such as Arias intensity, peak acceleration, peak velocity, Fourier spectra, and response spectra. The methodology proposed in this paper can be adapted to different databases provided that regional GMMs for Arias intensity and significant duration are available.

Supplementary Information The online version contains supplementary material available at <https://doi.org/10.1007/s10518-021-01077-1>.

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Availability of data and material The updated table of ITA18 coefficients for Joyner-Boore distance is provided as supplementary electronic material.

Code availability The code supporting the findings of the current study is publicly available in the Mendeley data repository with the identifier: <https://doi.org/10.17632/vnsb4jgjm.1>.

Declarations

Conflict of interest The authors declare no conflict of interest.

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